## Signal Coordination for Arterials and Networks



## Progression: Why Needed

$>$ Where signals are close enough together
$>$ Vehicles arrive in platoons
$>$ It is necessary to coordinate their times so that vehicles may move efficiently through set of signals

## Signal Spacing and Progression

$>$ In some cases signals are so closely spaced that they should be considered as one signal
$>$ In other cases, signals are so far apart that they may be considered as isolated
$>$ Common practice is to coordinate signals less than a mile apart on major streets and highways

## Key Requirement

$>$ All intersections in system are to have the same cycle length or multiple of minimum cycle length

Long enough to provide sufficient capacity at the busiest intersection
$>$ System cycle length is determined through a series of steps
\$ determine the minimum (optimum) cycle length at each intersection

- as if they are isolated signals


## Time Space Diagram

* Path a vehicles takes as time passes
* $t=t_{1}$
- First signal turns green
* $\mathrm{t}=\mathrm{t}_{2}$
- Vehicle reaches second signal
* Offset: Difference between green initiation times ( $\mathrm{t}_{2}-\mathrm{t}_{1}$ )



## Lecture 12

## Offset

\$ Ideal offset is defined as time needed for the first vehicle of the platoon just arrives at the downstream signal when it's green
$\mathrm{t}_{\text {ideal }}=\frac{\mathbf{L}}{\mathbf{S}}$

- tideal: ideal offset, sec
- L: Distance between signalized intersections, ft
- S: Vehicle speed, ft/sec

Offset is a positive number between zero and cycle length

## Offset is critical

$>$ Ideal offset 25 sec
$>$ See the effect of poor offset
$>$ An offset of $(25+10=35 \mathrm{sec})$

- Causes more harm than
- an offset of $(25-10=15 \mathrm{sec})$

(a) Stops

(b) Delay

600 ft block
600 vph in two lanes
all through traffic
free speed 24 mph
50-50 split
60 sec. cycle length

## Bandwidth

$>$ Defined as the amount of green time that can be used by a continuous moving platoon of vehicles through a series of intersections
$>$ In the example, previously,

- The green time at both intersections are same
- The ideal offset is illustrated
- There are only two intersections



## Signal Progression of One-Way St.

Assumptions:

1. Desired platoon speed: $60 \mathrm{ft} / \mathrm{sec}$;
2. Cycle length $=60 \mathrm{sec}$
3. Effective green time $=30 \mathrm{sec}$


Table 26.1: Ideal Offsets for Case Study

| Signal | Relative to Signal | Ideal Offset |
| :---: | :---: | :---: |
| 6 | 5 | $1,800 / 60=30 \mathrm{~s}$ |
| 5 | 4 | $600 / 60=10 \mathrm{~s}$ |
| 4 | 3 | $1,200 / 60=20 \mathrm{~s}$ |
| 3 | 2 | $1,200 / 60=20 \mathrm{~s}$ |
| 2 | 1 | $1,200 / 60=20 \mathrm{~s}$ |

TSD for One-way Street Example


## Bandwidth



## Potential Problems (Vehicle Speed)



## Bandwidth Concepts

Definition again: Defined as the time difference between the first vehicle that pass through the entire system without stopping and the last vehicle that can pass through without stopping, measured in seconds.

## Bandwidth Efficiency

$>$ The efficiency of bandwidth is defined as the ratio of the bandwidth to the cycle length, expressed as percentage
$E E F_{B W}=\left(\frac{B W}{C}\right) * 100 \%$

- EFFBW: Bandwidth efficiency (\%)
- BW: Bandwidth, sec
- C: Cycle length, sec

Bandwidth efficiency of $40 \%$ to $55 \%$ is considered good

Example: Bandwidth Efficiency
$\checkmark$ NB: $(17 / 60) * 100=28.4 \%$
$\checkmark \mathrm{SB}=0$


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## Bandwidth Capacity

$\$$ Defined as the number of vehicles that can pass through the system without stopping in one hour

* In the previous example:
- Consider a saturation headway of $2.0 \mathrm{sec} / \mathrm{veh}$
- Vehicles can pass per cycle: $17 / 2=8.5 \mathrm{veh} /$ cycle
- Thus NB direction can handle
[8.5 (veh/cycle) $]^{*}[1 / 60 \mathrm{cycle} / \mathrm{sec}] *[3600 \mathrm{sec} / \mathrm{hr}]$ $=510 \mathrm{veh} / \mathrm{hr}$
* If the per lane demand volume is less than 510 vphpl and if the flow is well organized the system will operate well in NB direction (even though better timing plans may be obtained)
$E E F_{B W}=\left(\frac{3600 * B W * N L}{C * h}\right)$
- $\mathrm{C}_{\mathrm{BW}}$ : Bandwidth capacity
- BW: Bandwidth, sec
- C: Cycle length, sec
- NL: Number of directional through lanes
- h: Saturation headway, sec

Note: The above equation does not contain any factors to account for lane utilization and queuing

## Effect of Queued Vehicles


$\mathbf{t}_{\mathrm{adj}}=\frac{\mathbf{L}}{\mathbf{S}}-\left(\mathbf{Q h}+\mathbf{l}_{\mathbf{1}}\right)$
$t_{\text {adj: }}$ : Adjusted ideal offset, sec
$>$ L: Distance between signals, ft
$>$ S: Speed, ft/sec
$>$ Q: Number of queued vehicles per lane, veh
$>\mathrm{H}$ : Discharge headway of queued vehicles, sec/veh -
$1_{1}$ : Start-up lost time, sec

Note: The start-up lost time is only accounted for first downstream intersection.

- If the preceding intersections have queue, then they are automatically cleared (because of platoon)

Calculate offset if
$\checkmark 2$ queue veh/cycle
$\checkmark$ Sat. headway 2 sec
$\checkmark$ Lost time, 2 sec


(2) $=2$ Vehicles per lane queued

| Link | Link Offset (s) | Speed of Progression (ft/s) |
| :---: | :---: | :---: |
| Signal $1 \rightarrow 2$ | $(1,200 / 60)-(4+2)=14$ | $1,200 / 14=85.7$ |
| Signal $2 \rightarrow 3$ | $(1,200 / 60)-(4)=16$ | $1,200 / 16=75$ |
| Signal $3 \rightarrow 4$ | $(1,200 / 60)-(4)=16$ | $1,200 / 16=75$ |
| Signal $4 \rightarrow 5$ | $(600 / 60)-(4)=6$ | $600 / 6=100$ |
| Signal $5 \rightarrow 6$ | $(1,800 / 60)-(4)=26$ | $1,800 / 26=69.2$ |

Total Offset $=78 \mathrm{sec}$

## Example

Two signals are spaced in 1000 ft . The desired progression speed is 40 mph , cycle length is 60 sec,

- and What is the ideal offset between two intersections


## Signal Progression on Two-way St

If NB offset applied to SB

* Vehicles need to stop twice
* Lesser bandwidth
* About 40 sec of delay/veh
* There is no bandwidth


Offsets in Two-way Street

(a) Offsets Add to One Cycle Length

(b) Offsets Add to Two Cycle Lengths

Two-way Street with 4 -Signals


## Adding a Fifth Signal



Lecture 12

## Adding a Signal with Diff. Cycle Length



Example
Distance ( ft )


## Common Types of Progression

$>$ The alternate progression
$>$ The double alternate progression
> The simultaneous progression
For certain uniform block lengths, and al intersections with a $50-50$ split of effective green time, the cycle length can be selected such that:
$\frac{C}{2}=\frac{L}{S}$

## Bandwidth Capacity

$C_{B W}=\frac{3600 * B W * N L}{C * h}=\frac{3600 * 0.5 C * N L}{C * 2}=900 N L$


## Illustrative Combination of Alternate Progression

| Cycle Length (s) | Platoon Speed (fps) | Matching Block <br> Length (ft) |
| :---: | :---: | :---: |
| 60 | 45 | 1,350 |
| 60 | 75 | 2,250 |
| 90 | 45 | 2,025 |
| 90 | 75 | 3,375 |

## Double Alternate Progression

For certain non-uniform block lengths, $50-50$ split of effective green time is not possible, but the it is feasible to select the following cycle length
$\frac{C}{4}=\frac{L}{S}$

## Bandwidth Capacity

$C_{B W}=\frac{3600 * B W * N L}{C * h}=\frac{3600 * 0.25 C * N L}{C * 2}=450 N L$


Illustrative Combination of Double Alternate Progression

| Cycle Length (s) | Platoon Speed (fps) | Matching Block <br> Length (ft) |
| :---: | :---: | :---: |
| 60 | 45 | 675 |
| 60 | 75 | 1,125 |
| 90 | 45 | 1,012 |
| 90 | 75 | 1,688 |

## Simultaneous Progression

For very closely spaced signals, or for higher vehicle speeds, it may be best possible to have all the signals turn green at the same time (called as simultaneous system)

$>$ The efficiency of simultaneous system depends on number of signals involved.
$\mathrm{EFF} \%=\left[\frac{1}{2}-\frac{(\mathrm{N}-1) * \mathrm{~L}}{\mathrm{~S} * \mathrm{C}}\right] * 100$
$>$ For four signals with $\mathrm{L}=400 \mathrm{ft}, \mathrm{C}=80 \mathrm{sec}, \mathrm{S}=45 \mathrm{ft} / \mathrm{sec}$

- What is EFF\%?
$>$ Similarly when $\mathrm{L}=200 \mathrm{ft}, \mathrm{EFF} \%$ ?
* Simultaneous signals are advantageous only under special circumstances
- Block length are very short
- Relatively higher speed
- Near CBD


## Lecture 12

* Disadvantages
- May result breakdown and spillback
- Because queue inevitably exists in downstream
- Cuts platoons off


## Speed versus Congestion

- Speed of progression
- often selected to be the free-flow speed of traffic
- Some engineers use it as speed limit.
- speed that might be observed when volumes are light and the signals are continuously green on that route
As traffic becomes heavy (peak periods)
$\checkmark$ traffic speeds tend to drop because of congestion
$\checkmark$ traffic starting up on a green signal may be stopped by a queue not yet into motion at the next signal downstream



## Other considerations

$>$ Traffic Turning into System
> Adjustments at End Intersections
$>$ Adjustments for Left-Turn Phases
$>$ Offsets for Maximum Bandwidths
$>$ Offsets for Minimum Stops and Delay

## Softwares

Relatively Smaller Networks

- PASSER II- Optimize bandwidths
- TRANSYT 7F- Minimizes disutility function
- NETSIM- Optimizes synchronization
- Synchro- models and optimizes traffic signals

Optimizes to reduce delay

- SimTraffic- to check and fine tune signal operations

Medium to Large Networks

- AIMSUN, VISSUM, TransModeler, Paramics, and others

