

3- Design of Alluvial canals Using Tractive force Method

canals of erodible material can be designed to prevent the expected scouring. This Method is based on the concept of tractive force. ^{نظم القوت التردية بأستزاد من مبدأ (قوى الجريان)}
 The scour in a canal occurs when tractive force exerted by flow is adequate to cause movement of bed particle having representative diameter d_s or d_{50} . This method is proposed by Lane also known as the USBR method. ^{النشأ كل بحيث عندما تكون قوى الجريان كافية قبل الجزيئات لتؤديه جزيئات قاع القناة التي لها d_{50} معين}

$$P_1 - P_2 - F + W \sin \alpha = \rho Q (V_2 - V_1)$$

↑ التغيير بالزمن = $\frac{dP}{dt}$ القوة

P : pressure V : velocity
 F : Resultant force W : weight of soil particle

ρ : density Q : discharge

For uniform flow ^{جريان منتظم}

$$\therefore P_1 = P_2, \quad V_1 = V_2$$

أي لا يوجد تغيير في السرعة والعمق

$$\therefore F = W \sin \alpha$$

$$F = \tau_b * P * L \quad \left[\begin{array}{l} \text{حيث أن } F \text{ هي القوة التي تميل القاع المحركة} \\ \text{لجزيئات القاع باتجاه الجريان} \end{array} \right]$$

where τ_b : shear stress at level of bed

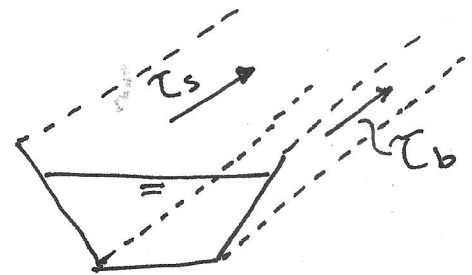
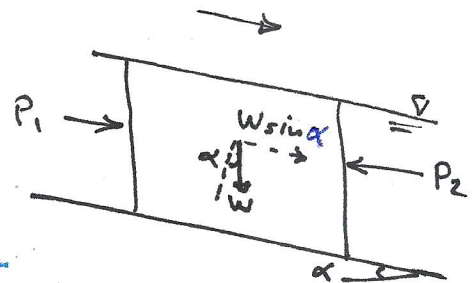
τ_s : = = = side slope.

$$\therefore \tau_b P L = W \sin \alpha$$

$$\tau_b P L = (\gamma A L) \tan \alpha$$

$$\tau_b P L = \gamma A L S$$

$$\boxed{\tau_b = \gamma R S}$$



[since α is small
 $\therefore \sin \alpha = \tan \alpha$]

From some relations, it has been proved that :

$$\frac{\tau_s}{\tau_b} = \sqrt{1 - \frac{\sin^2 \theta}{\sin^2 \phi}}$$

where :

$$K = \sqrt{1 - \frac{\sin^2 \theta}{\sin^2 \phi}}$$

K : tractive ratio

θ : side slope $\left[\tan^{-1} \left(\frac{1}{z} \right) \right]$

ϕ : angle of repose زاوية الاحتكاك الداخلي
أو زاوية الاستقرار

$$\therefore \tau_s = K \tau_b$$

In design: τ_b should be less than critical shear stress τ_c

$$\tau_b < \tau_c$$

$$\text{let say } \tau_b < 0.9 \tau_c$$

τ_c can be calculated from different approximated methods
 τ_c is strongly related to particle size d . But the more actual value of τ_c , can be obtained from shields diagram shown in fig below (2) also it can be obtained using fig (3) which is adapted form of shields curve for incipient motion [by Yalin & Karahan]

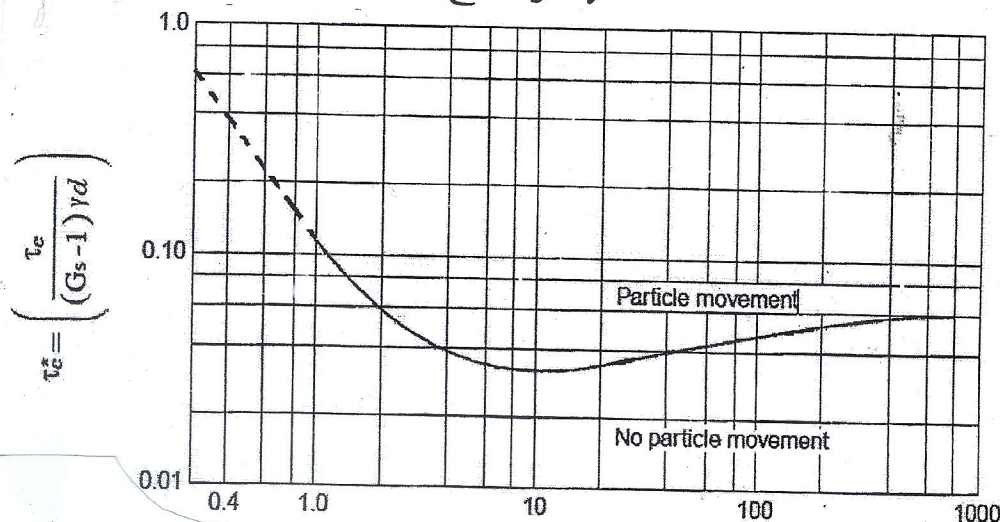


Fig (2) shields curve for incipient motion

$$R_c^* = \left(\frac{V_{*c} d}{\nu} \right)$$

والاخير هو الذي
يستخدم ليجاد
 τ_c

where V_* : shear velocity

$$V_* = \sqrt{\tau_b / \rho}$$

R_c^* : dimensionless
Reynolds no. or
particle Reynolds no.

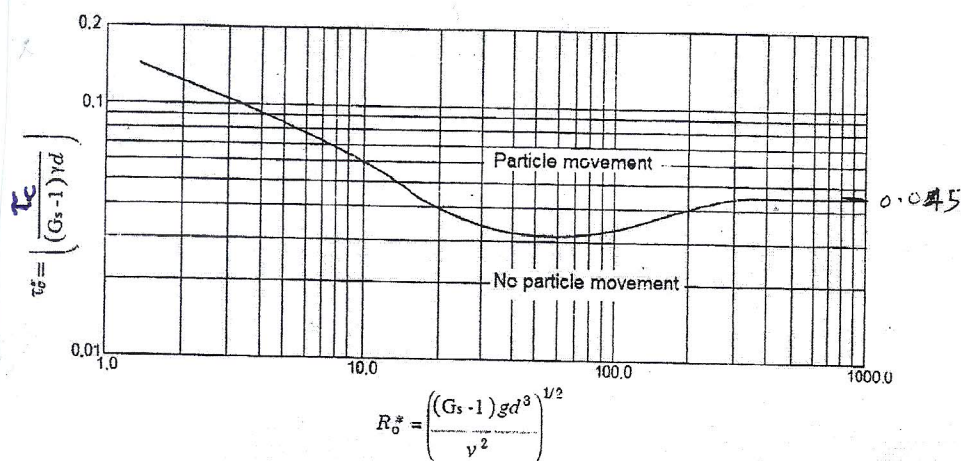


Fig. (3) Yalin and Karahan curve Adapted from Shields curve for incipient motion

where :

R_o^* : boundary Reynolds No.

How to find τ_c :

- ① $G_s = \frac{\gamma_s}{\gamma}$
- ② for given Kinematic viscosity (ν) of water $1 \times 10^{-6} \frac{m^2}{s}$
- ③ from fig find dimensionless critical shear stress τ_c^*
- ④ $\tau_c = \tau_c^* (G_s - 1) \gamma \cdot d$

How to design

- ① for a given d (particle diam.), γ_s (soil density), γ , ν (Kinematic viscosity)
- ② $\tau_{b_{max}} = 0.9 \tau_c$
- ③ $\theta = \tan^{-1} \frac{1}{2}$
- ④ ϕ coefficient of friction (angle of repose), calculate $K = \sqrt{1 - \frac{\sin^2 \theta}{\sin^2 \phi}}$
- ⑤ $\tau_{s_{max}} = K \tau_{b_{max}}$
- ⑥ Assume $\frac{B}{\gamma}$ ratio
- ⑦ from fig below (4) find $\frac{\tau_{s_{max}}}{\gamma_{ys}} \Rightarrow$ find γ

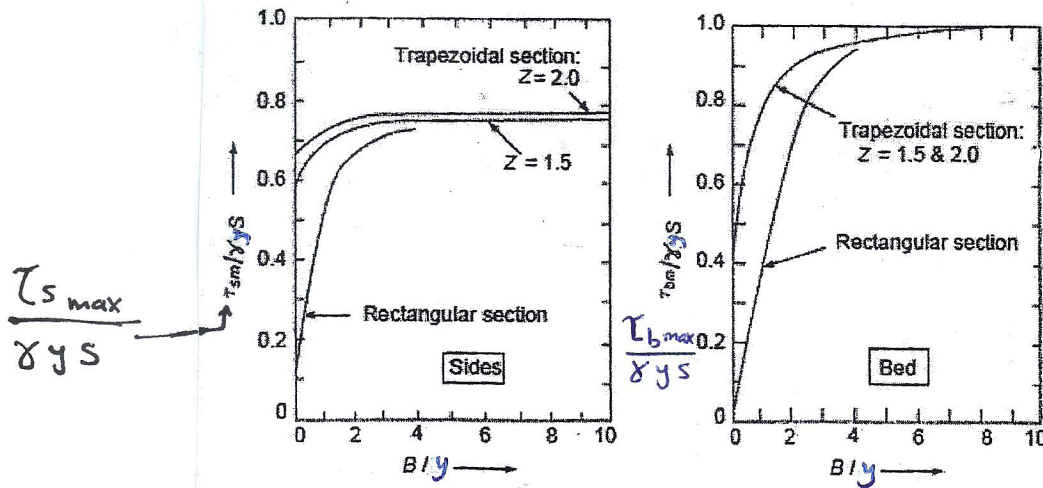


Fig. 4 Variation of maximum shear stress on bed and sides of smooth channels

go to step ⑥ $\uparrow$ 90 Find B
 ⑥ $\rightarrow$ ⑩ for given Q using Manning equation to check $y \neq B$.

Ex: Design a trapezoidal canal ($Z=2$) to carry $25 \frac{m^3}{s}$
 The slope $S = 1 \times 10^{-4}$. The bed and banks comprise gravel (angle of repose $\phi = 31^\circ$) and the size is 3mm.
 Use $\nu = 10^{-6} m^2/s$ & $n = 0.0148$.

Sol.

$$R_o^* = \left[\frac{(G_s - 1) g d^3}{\nu^2} \right]^{1/2} = \left[\frac{2.65 - 1 (9.81) (0.003)^3}{1 \times 10^{-12}} \right]^{1/2}$$

$$R_o^* = 661.1 \quad \text{from fig (3)} \quad \tau_c^* = 0.045$$

$$\tau_c = 0.045 (2.65 - 1) 9810 (0.003) = 2.185 N/m^2$$

$$\tau_b = 0.9 \tau_c = 1.97 N/m^2 = \tau_{b \max}$$

$$\theta = \tan^{-1} \frac{1}{2} = 26.56^\circ$$

$$\tau_b * K = \tau_s \Rightarrow \tau_s = 1.97 \sqrt{1 - \frac{\sin^2 26.56^\circ}{\sin^2 31^\circ}} = 0.977 N/m^2$$

assume $\frac{B}{y} = 10$ since $Q > 10 m^3/s$ [Page 9]

from fig (4)

$$\frac{\tau_{s \max}}{\gamma y S} = 0.78 \Rightarrow y = \frac{0.977}{9810 * 1 * 10^{-4} * 0.78} = 1.27 m$$

$$B = 10 y = 12.7 \text{ m}$$

$$\left. \begin{aligned} A &= By + 2y^2 = 19.355 \text{ m}^2 \\ P &= B + 2\sqrt{5} y = 18.38 \end{aligned} \right\} R = 1.053 \text{ m}$$

$$Q = \frac{1}{n} A R^{2/3} S^{1/2}$$

$$= \frac{1}{0.0148} (19.355) (1.053)^{2/3} (10^{-4})^{0.5}$$

$$= 13.5 \text{ m}^3/\text{s} < 25 \text{ m}^3/\text{s} \Rightarrow \text{choose } \frac{B}{y} = 20$$

$\frac{B}{y}$	$\frac{\tau_{s \max}}{\gamma y s}$		B	A	P	R	Q
10	0.78	1.27	12.7	19.355	18.38	1.053	13.5
20	.78	1.27	25.4	35.484	32.08	1.142	26.194

H.W Design the cross sectional of trapezoidal channel carrying discharge of $1.6 \text{ m}^3/\text{s}$ if $s = 30 \text{ cm/km}$. bed material is silty loam, use $n = 0.025$, $\phi = 36.3^\circ$